

Estimating Probability Distribution of Benefit from Flood Control Projects

Abstract—Frequencies of severe flood events are expected to increase due to a changing climate. Therefore, it becomes more and more important to implement suitable flood defenses based on quantitative benefit analysis of flood control projects such as dam construction and embankment. In general, the benefit from flood control projects is defined as the sum of expected value of annual reduced damage cost over the evaluation years, considering discount rate of each year. Since flooding is low-frequency events especially in highly protected areas, it is highly uncertain whether actual reduced damage cost is distributed around its expected value, i.e. benefit calculated from the expected value does not necessarily occur frequently. The probability distribution of reduced damage cost would provide more comprehensive information for decision making. This study redefined benefit from a flood control project as a reduced damage cost, which is actually a random variable, and developed its probability distribution by applying an extreme rainfall generation method. Then, we demonstrated the presented method under several scenarios of dam construction projects. The estimated probability distribution was skewed leftwards, and had a long tail. It reveals large variability of reduced damage cost. From the probability distribution, we can extract several statistics in addition to the expected value, which provide more information to discuss the characteristics of actual reduced damage cost by flood control projects.

Keywords— *benefit from flood control projects; probability distribution; uncertainty;*

I. INTRODUCTION

It is concerned that frequencies of intense rainfall events will increase due to climate change. To implement adaptation measures against the expected severe flood events, countermeasures should be taken, and fundamental measures are constructing flood defenses such as embankment and dam. In Japan, cost-benefit analysis is utilized from 1997 to evaluate the efficiency of public works, where benefit corresponds to reduced inundation damage cost by installing flood defenses, and the efficiency of flood control projects are measured by comparing it with the cost for construction and maintenance. According to the manual issued by the Ministry of Land, Infrastructure, and Transport (hereinafter called “MLIT”)[1], the benefit from flood control projects is defined as the sum of the expected value of annual reduced damage cost over the evaluation years, considering discount rate of each year.

Theoretically, annual reduced damage cost is actually a random variable; accordingly, the sum of annual reduced damage cost is also random variable. Therefore, the benefit defined above is equivalent to the expected value of the sum of annual reduced damage cost over the evaluation years when neglecting discount rate. According to central limit theorem, actual reduced damage cost is normally distributed around the benefit calculated from the expected value when evaluation period is long enough. However, this is not guaranteed for an actual evaluation period due to the low frequency of flooding. In other words, it is highly uncertain whether actual reduced damage cost is distributed around its expected value, i.e. benefit calculated from the expected value does not necessarily occur frequently. These facts show that the above deterministic approach to decision making of flood control projects based on cost-benefit analysis includes the uncertainty of the probabilistic characteristics of reduced damage cost. If we can estimate the probability distribution of reduced damage gained over evaluation period, we can get more comprehensive information from the distribution in addition to the expected value, e.g. the probability with which actual reduced damage cost becomes larger than expected value. These will help advanced decision making of flood control projects based on multiple criteria.

In a literature, several uncertainties for benefit calculation have been discussed, e.g. the uncertainty of risk evaluation process including flood frequency analysis [3] and risk premium. For example, in the field of urban planning, it is discussed that willingness to pay becomes higher than the expected value of reduced damage cost because of risk aversion [4]. Risk premium is induced from the probability distribution of reduced damage cost; therefore, estimating the probability distribution of benefit is important to explicitly incorporate the idea of risk premium into in the process of decision making for disaster prevention investment.

On the other hand, recent studies have developed methods for estimating a flood risk curve, i.e. the probability distribution of annual maximum inundation damage cost, considering influential factors of risk evaluation such as rainfall time-space distribution and/or hydrograph variations [2],[5]. For instance, Tanaka et al. [5] showed that considering the rainfall time-space distribution makes it possible to enhance the accuracy of evaluating flood disaster risk. This study demonstrates the derivation of the probability distribution of T -year cumulative reduced-damage cost by generating huge number of rainfall events in consideration of rainfall time-space distribution.

From these backgrounds, the purpose of this study is estimating the distribution of T -year cumulative reduced damage which is defined as sum of annual reduced damage cost over T years, and quantifying the probability with which

we can actually benefit from flood control projects. The study area is Yodo River basin (8,240km²) and several construction scenarios of the existing dams in the Kizu River basin (Takayama Dam, Murou Dam, Shorenji Dam, Hinachi Dam, Nunome Dam) are evaluated. For each project, we estimate the probability distribution of T -year cumulative reduced damage cost at conjunction area of three tributaries, Kizu River, Uji River, and Katsura River, and analyze its stochastic characteristics. The impact of design of projects and evaluation period on the estimated distribution is also examined.

II. BASIC CONCEPT OF ESTIMATING PROBABILITY DISTRIBUTION OF T-YEAR CUMULATIVE REDUCED DAMAGE COST

According to the manual of cost-benefit analysis issued by MLIT[1], benefit of flood defenses over T years $B(T)$ is defined as

$$B(T) = \sum_{t=1}^T \frac{b}{(1+r)^{t-1}}, \quad (1)$$

where b is expected value of annual reduced damage; r is discount rate; and T is evaluation period. Denoting annual flooding damage before and after flood control investment as D_a and D_b , respectively, b is defined with annual reduced damage $D_R = D_b - D_a$ as

$$b = E[D_R] = E[D_b] - E[D_a]. \quad (2)$$

On the other hand, we define T -year cumulative reduced damage cost $D_{R,all}(T)$ as follows.

$$D_{R,all}(T) = D_{b,all}(T) - D_{a,all}(T) \quad (3)$$

where $D_{b,all}(T)$ and $D_{a,all}(T)$ are cumulative damage cost before and after investment, and denoted as

$$D_{b,all}(T) = \sum_{t=1}^T D_{b,t}, \quad D_{a,all}(T) = \sum_{t=1}^T D_{a,t}, \quad (4)$$

respectively, where $D_{b,t}$ and $D_{a,t}$ are flooding damage costs of year t before and after flood control investment. In this study, we estimate probability distribution of T -year cumulative reduced damage cost $D_{R,all}(T)$ defined in (3). In addition, we investigate impact of evaluation period T length on the probability distribution of $D_{R,all}(T)$. Note that expected value of T -year cumulative reduced damage cost is obtained from (1) and (4), and we can confirm that benefit $B(T)$ used in the manual [1] corresponds to the mean value of $D_{R,all}(T)$ when neglecting discount rate, i.e.

$$\begin{aligned} E[D_{R,all}(T)] &= E[D_{b,all}(T)] - E[D_{a,all}(T)] \\ &= \sum_{t=1}^T E[D_{b,t} - D_{a,t}] = \sum_{t=1}^T E[D_{R,t}] = \sum_{t=1}^T b, \end{aligned} \quad (5)$$

where $D_{R,t}$ is reduced damage cost of year t .

III. METHODOLOGY OF ESTIMATING PROBABILITY DISTRIBUTION

In this research, we take a Monte Carlo approach to obtain enough number of samples of T -year cumulative reduced damage cost, by generating enormous number of extreme rainfall events. Assume rainfall events follow the following three assumptions which were used by Tanaka et al. [5].

Assumption 1. When rainfall occurs, possible rainfall pattern is limited to the N patterns. Pattern ξ_i ($i = 1, 2, \dots, N$) occurs with probability p_i . Denote rainfall duration time of pattern ξ_i as d_i .

Assumption 2. Basin averaged total rainfall r_a follows the conditional cumulative distribution function (CDF) $G_{R_a|D}(r_a|d_i)$ given d_i .

Assumption 3. Annual number of occurrences of rainfall events follows the Poisson distribution with occurrence ratio of μ_a .

Define $r(x, y, t)$ as rainfall intensity at a location (x, y) and time t , and rainfall pattern ξ_i is defined as

$$r(x, y, t) = r_a \xi(x, y, t), \quad (x, y) \in A, 0 \leq t \leq d, \quad (6)$$

where A is the targeted watershed. $\xi(x, y, t)$ is normalized to satisfy the following equation:

$$\iint_A \left(\int_0^d \xi(x, y, t) dt \right) dx dy = S, \quad (7)$$

where S means the catchment area of targeted watershed. Rainfall duration time d is defined as follows.

$$d = \sup\{t | \xi(x, y, t) > 0, \exists (x, y) \in A, t > 0\}. \quad (8)$$

Defining the starting time of rainfall as 0, d indicates the maximum time until which rainfall lasts at a certain point in the watershed.

Following these assumptions, we generate immense samples of T -year cumulative reduced damage cost, following the steps shown below.

Step 1. Generate the annual number of occurrences of rainfall events n from the Poisson distribution

Step 2. Prepare N past rainfall events and extract rainfall patterns $\xi_i(x, y, t)$ normalized by total rainfall amount. Note that t satisfies inequality, $0 \leq t \leq d_i, (x, y) \in A$ where d_i is the rainfall duration time of pattern $\xi_i(x, y, t)$; A is the catchment area. Generate one pattern with the probability $1/N$, i.e. randomly.

Step 3. By following Step 2, the conditional CDF $G_{R_a|D}(r_a|d_i)$ corresponding with the generated rainfall pattern $\xi_i(x, y, t)$ is determined. Generate total rainfall amount r_a from the conditional CDF $G_{R_a|D}(r_a|d_i)$. From the obtained total rainfall amount r_a and its spatiotemporal pattern $\xi_i(x, y, t)$, one rainfall event is determined

Step 4. Running a rainfall-runoff model and an inundation model, calculate damage costs with/without investment, D_b and D_a , f.Then, calculate the reduced damage cost for the generated rainfall event.

Step 5. Repeating Steps 2 to 4 for n times, sum reduced damage costs of each rainfall events and obtain one sample of annual reduced damage cost D_R .

Step 6. Repeating Steps 1 to 5 for T times, sum annual reduced damage and get one sample of T -year cumulative reduced damage cost $D_{R,all}(T)$. In this research, discount rate was eliminated to focus the discussion on probabilistic characteristics of reduced damage cost.

Step 7. Repeat Step 6 for M times and estimate the probability distribution of T -year cumulative reduced damage cost from the many samples of $D_{R,all}(T)$.

IV. APPLICATION TO THE DAM CONSTRUCTION PROJECTS

A. Rainfall generation and Damage cost calculation

The above presented method was applied to four different scenarios of dam construction projects for five actual dams along the Kizu River (Takayama Dam, Murou Dam, Shorenji Dam, Hinachi Dam, Nunome Dam), whose locations are displayed in fig. 1. According to Tanaka et.al.[6], when flood beyond design level occurs, the river overflows around the three tributaries' conjunction area, and the overtopping probability of downstream Yodo River becomes very low. Hence we analyze the effect of dams from the reduced damage cost at three tributaries' conjunction area indicated by the red colored box in fig.1. In the flood simulation, we do not consider dyke breach, and only consider damage from overtopping.

We used the conditional CDF of basin-averaged rainfall on duration $G_{R_a|D}(r_a|d_i)$ on the Yodo River basin and rainfall-runoff/inundation models estimated/constructed by Tanaka et.al. [6] as follows. Rain gauge data for 35 years (1980 to 2014) at 156 stations were used in analysis. They extracted spatiotemporal rainfall patterns of the 1,317 rainfall events observed over 35 years, i.e. $N = 1,317$. From the basin-averaged rainfall and duration time of the 1,317 rainfall events over 35 years, they estimated the conditional probability distribution of one basin-averaged rainfall on duration time. One rainfall event is defined as a series of rainfall events without dry period longer than 5 hours. In order to estimate conditional probability distribution, the generalized Pareto distribution and two-variable exponential distribution were fitted to the marginal distribution of basin-averaged rainfall $G_{R_a}(r_a)$ and rainfall duration time $G_D(d)$, respectively. Their correlation structure $C(G_{R_a}(r_a), G_D(d))$ was modelled by using the normal copula. According to the Theorem of Sklar[7], this notation indicates the simultaneous distribution $G_{R_a,D}(r_a, d)$. Finally, the conditional cumulative distribution was obtained as

$$G_{R_a|D}(r_a|d) = \frac{1}{g_D(d)} \frac{\partial}{\partial d} G_{R_a,D}(r_a, d). \quad (9)$$

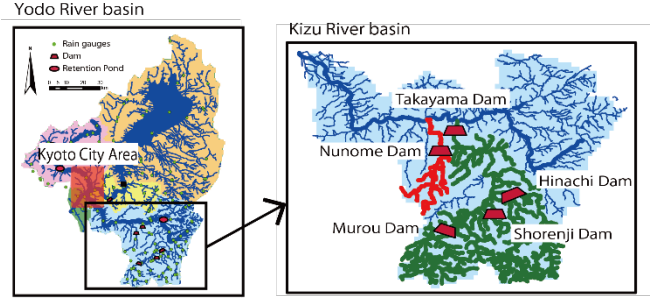


Fig. 1. Figure of Yodo River basin.

As a rainfall-runoff model used was 1K-DHM[8], a distributed rainfall runoff model based on a kinematic wave flow approximation. 1K-DHM calculates the slope runoff and river routing at each cell at around 1km (30 sec) resolution over a two dimensional domain. Slope runoff is simulated with the continuity and momentum equations of the kinematic wave theory, considering saturated and unsaturated subsurface flows and surface flow with the following discharge-storage relation.

$$q = \begin{cases} v_c d_c \left(\frac{h}{d_c}\right)^\beta & (0 \leq h \leq d_c) \\ v_c d_c + v_a (h - d_c) & (d_c \leq h \leq d_a) \\ \alpha (h - d_c)^m + v_a (h - d_c) + v_c d_c & (d_a \leq h) \end{cases} \quad (10)$$

where, q is the slope runoff discharge per unit width; h is the water depth; d_a and d_c are the water depth corresponding to the maximum water content of saturated and unsaturated soil layers, respectively; v_a and v_c are the water velocity of saturated and unsaturated soil layers, respectively; $\alpha = n/\sqrt{\sin\theta}$; n is the roughness coefficient; and θ is the slope gradient. River flow is simulated with a kinematic wave model. All dams in Yodo basin are modeled in accordance with their operation rules, and rainfall-runoff before/after a project is represented by excluding/including the corresponding dam models.

Overflow inundation over the floodplain was modelled with one dimensional local inertial equation for river routing and two dimensional one for floodplain simulation. One dimensional local inertial equation is denoted as

$$\frac{\partial A}{\partial t} + \frac{\partial Q}{\partial x} = 0, \quad (11)$$

$$\frac{\partial Q}{\partial t} + gA \frac{\partial(h+z)}{\partial x} + gn^2 \frac{Q|Q|}{R^{4/3}A} = 0, \quad (12)$$

where t is the time; x is the location in the downstream direction; A is the cross-sectional area; Q is the discharge; z is the elevation; g is the gravity acceleration; n is the roughness coefficient; and R is the hydraulic radius. Neglecting the advective term of the Saint-Venant equation, high speed arithmetic operation is realized with the same level of accuracy as the diffusion equation. Damage cost is calculated for house, households, and office depreciable/stock assets, by multiplying exposed asset at each cell of the inundation model by damage ratio which is a function of maximum flood depth. The function is defined in the manual of MLIT [1].

In accordance with MLIT's manual[1], evaluation period T was set as 50 years. When disaggregating a rainfall event into to basin-averaged cumulated rainfall and its spatiotemporal pattern as in this study, it is obvious that flood damage cost monotonically increases with basin-averaged rainfall for a fixed rainfall pattern; therefore, we have developed a relation between cumulated rainfall and resulting flood damage cost for all the rainfall patterns before Monte Carlo simulation, then in Step 4 to calculate flood damage cost, flood damage cost for the generated rainfall event was calculated by linearly interpolating the relation for the generated rainfall pattern, instead of running rainfall-runoff/inundation models for all the generated events. Repeating the calculation of T -year cumulated reduced damage cost for $M = 100,000$ times, we estimated the probability distribution of the benefit of each scenario.

B. Trial Calculation Results of 50 years total reduced damage cost.

Among the five dams, we first set project J: there is no dam on the Kizu River and construct Takayama Dam on the basin. Figure 2 shows the histogram of simulated T -year cumulated reduced damage cost. Red line indicates the expected value. JPY is converted to USD at a rate of 100 yen per dollar. Obviously, the distribution is skewed leftwards with a long tail. The probability with which the benefit is equal to or over the expected value is about 10-20%, and one with which benefit is obtained is about 55%. On the other hand, extremely large benefit values much larger than the median value occurs with a certain probability. These indicated that the expected value integrated different types of benefit and made it difficult to grasp the whole characteristic of the benefit of the project. By estimating the distribution of reduced damage cost as fig.2, more detailed information as demonstrated above becomes available, which is expected to support more rational decision making.

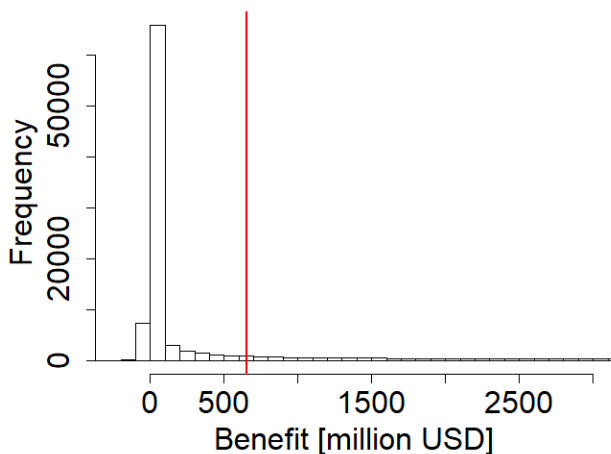


Fig. 2. Histogram of 50-year cumulated reduced damage cost of the project constructing Takayama Dam. Red line indicates expected value.

V. DIFFERENCES OF PROBABILITY DISTRIBUTION BETWEEN PROJECTS

Among the five target dams, the Takayama Dam has the largest flood storage capacity. As shown in fig.1, among the other four dams, Nunome Dam is located on a different tributary river. To examine the impact of the location of dams on the estimated probability distribution, we split the construction project of the four dams into projects K, and project L: Project K first constructs three dams except for the Nunome Dam (Shorenji Dam, Murou Dam, and Hinachi Dam), and Project L then constructs the Nunome Dam. Table.1 shows the statistics of three projects (J, K, L) gained from their probability distribution of 50-year cumulated reduced damage cost. Comparing with Projects K and L, Project L shows lower skewness and higher probability of getting more benefit equal to or more than expected value, and the occurrence of benefit.

As shown in fig.1, in the Project K, Takayama Dam is already constructed in the same tributary, so dams constructed in Project K can mitigate damage only for flood causing inflow volume over the storage capacity of Takayama Dam. On the other hand, the Project L constructs Nunome Dam in the tributary where no dam has been constructed before; thus Nunome Dam more frequently shows flood control effect than dams installed by Project K. Accordingly, Project L is expected to benefit the target area more frequently than Project K. As shown in Table.1, difference between the expected value of Projects K and L is about 26%. On the other hand, the probability with which the benefit equal to or more than its expected value by Project L is 230% larger than Project K, indicating that using other statistics provided different characteristics of projects from the expected value.

VI. RELATION BETWEEN PROBABILITY DISTRIBUTION AND CENTRAL LIMIT THEOREM

When evaluation period T is large enough, probability distribution of T -year cumulated reduced damage cost theoretically converges to normal distribution, so reduced damage cost is assumed to distribute around its expected value. Figure.4 shows the variation of histogram of Project J when changing the evaluation period from 50 years to 1,000 years. For comparison of distribution characteristics, cumulated reduced damage cost was converged to annual

TABLE I. BENEFIT STATISTICS OF EACH PROJECT

Project	J	K	L
Expected value	655	197	157
Standard variaron	2240	804	355
Skewness	38.3	5.44	2.92
Probability with which benefit becomes larger than expected value	0.18	0.10	0.23
Occurrence probability of benefit	0.55	0.30	0.74

mean value by dividing evaluation year T . We can confirm that probability distribution of reduced damage cost skewed leftwards and have a long tail in limited evaluation period of 50 years, but the distribution approaches bilaterally symmetrical and the probability we can actually get benefit equal to or more than the expected value increases as the evaluation period becomes larger. This indicated that the general evaluation period of 50 years are not quite enough to represent the annual reduced damage cost by its expected value.

CONCLUSION

In the present benefit evaluation method, benefit is represented as the expected value of cumulative reduced damage in evaluation period. However, flooding is a low-frequency event, especially in highly protected areas; thus, it is highly uncertain whether actual reduced damage cost is distributed around its expected value. In this research, in order to quantify stochastic characteristics of benefit, we developed an estimation method of T -year cumulative reduced damage cost probability distribution, and applied the method to several scenarios of flood control projects on the Kizu River.

By estimating probability distribution of T -year cumulative reduced damage cost, we can get more statistics as well as mean value, and reveal the characteristics of each project. Following findings were obtained in this study:

1. In all the four projects, probability distribution skewed leftwards, and the probability with which we can actually obtain the benefit equal to or more than the expected value is about 10-20%. On the other hand, extremely large benefit values is expected with a certain probability. In the process of averaging reduced damage cost,
2. The expected value of benefit and other statistics such as the probability with which benefit is obtained show different characteristics depending on projects
3. When we set longer evaluation period, 1000 years, distribution of cumulative reduced damage approaches normal distribution, and benefit was evenly distributed around its expected value, compared with offsetting evaluation period to 50 years. This indicated that T -year cumulative reduced damage cost was distorted because the evaluation period was not long enough to represent the expected value as the benefit due to low frequency of flooding.

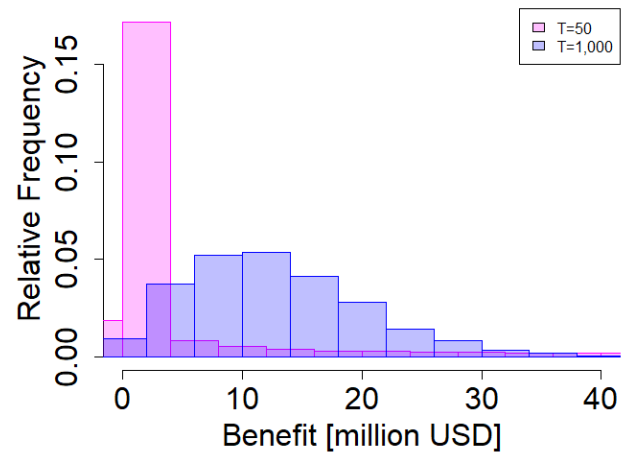


Fig. 3. Histogram of 50 and 1,000 years cumulative reduced damage cost of the project

This study proposed an idea to estimate the probability distribution of T -year cumulative reduced damage cost for providing information about various statistics of reduced damage cost and making more rational decision making. We will discuss how to incorporate this idea into the actual decision making process for advanced decision making process of flood control projects.

ACKNOWLEDGMENT

This study was supported by Kansai Energy Recycling Foundation.

REFERENCES

- [1] The flood control economic survey manual (draft) pp. 15-62, 2005.
- [2] Apel, H., Thicken, A. H., Merz, B. and Blöschl, G. "A probabilistic modelling system for assessing flood risks", *Nature Hazards*, Vol. 38, No. 1-2, pp. 79-100, 2006
- [3] Falter, D., Schröter, K., Dung, N. V., Vorogushyn, S., Kreibich, H., Hünedea, Y, Apel, H., Merz, B. "Spatially coherent flood risk assessment based on longterm continuous simulation with a coupled model chain. *Journal of Hydrology*, Vol. 524, pp. 182-193 2015.
- [4] Yokomatsu, M., Kobayashi, K. "Economic benefit of physical risk reduction by disaster prevention investment", No. 660/IV-49, pp.111-123, 2000.
- [5] Tanaka, T., Tachikawa, Y., Ichikawa Y., Yorozu, K. "A flood risk curve development using conditional probability distribution of rainfall on duration", *Journal of JSCE B1 (Hydraulic Engineering)*, Vol. 72, No. 4, I_1219-I_1224, 2016.
- [6] Tanaka, T., Ichikawa, Y., Yorozu, K., Tachikawa, Y. "Development of a probabilistic flood damage map and its application to benefit analysis of land raising", *Journal of JSCE Journal of JSCEB1 (Hydraulic Engineering)*, Vol. 72, No. 4, I_1477-I_1482, 2018.
- [7] Nelsen, R., 2006. *An Introduction to Copulas*. Springer, New York.
- [8] Tachikawa, Y., Tanaka, T.: 1K-DHM/1K-FRM, <http://hywr.kuciv.kyoto-u.ac.jp/products/1k-DHM/1k-DHM.html>